01. As per converse labarre formula group efficiency of friction piles is given by:

a.
$$1-rac{lpha}{90}\left[rac{(n-1)m+(m-1)n}{mm}
ight]$$

b.
$$1 - \frac{\alpha}{90} \left[\frac{(n+1)m + (m+1)n}{mm} \right]$$

C·
$$1 - \frac{\alpha}{90} \left[\frac{(n-1)m - (m-1)n}{mm} \right]$$

d.
$$1-\frac{\alpha}{90}\left[\frac{(n+1)m-(m+1)n}{mm}\right]$$

m = number of columns n = number of rows α = tan⁻¹ (d/s) (in degree) s = c/c spacing of pile

02. The correct increasing order of the surface areas of the given soils is

- a. Silt, sand, colloids, clay
- b. Sand, silt, colloids, clay
- c. Sand, silt, clay, colloids
- d. Clay, silt, sand, colloids

03. For a given soil sample, C_c = coefficient of gradation C_u = coefficient of uniformity D_{10} = effective size D₃₀ = diameter through which 30 per cent of the total soil mass is passing if $C_c = 1.0$ and $C_u = 4.0$ then the value of D_{30}/D_{10} would be

a. 2.00 c. 1.50

b. 1.75 d. 1.25

04. Match List I with List II and select the co answer using the codes given below the lists

given below the lists:
List II
(Field of contribution)
1. Flow through capillary
2. Classification of soils
3. Consistency limits
4. Flow of water through
a soil mass
5. Velocity of settling particle

- 05. In a standard proctor compaction, the water content (w) and maximum dry density (γ_{dmax}) are related as:
- a. γ_{dmax} is linearly proportional to w
- b. W is inversely proportional to $\gamma_{\rm dmax}$ c. $\gamma_{\rm dmax}$ corresponds to a unique value of w
- d. γ_{dmax} corresponds to w = ($w_p + w_L$)/2 where w_p and w_L are respectively plastic and liquid limits

06. Using Mohr's diagram, the relation between major principal stress $\boldsymbol{\sigma}_{\!\scriptscriptstyle 1}$ and minor principal stress $\sigma_{\!\scriptscriptstyle 3}$ and shear parameters C and ϕ is given by

 $\sigma_1 = \sigma_m N_\phi + 2c\sqrt{N}_\phi where N_\Phi is equal to$

- a. Sin ϕ / (1 + sin ϕ)
- b. Sin ϕ / (1 sin ϕ)
- c. $(1 \sin \phi) / (1 + \sin \phi)$
- d. $(1 + \sin \phi) / (1 \sin \phi)$
- 07. A and B are skempton's pore pressure parameters and $\Delta\sigma_{1}$ and $\Delta\sigma_{3}$ are incremental principal stresses. Skempton's pore pressure equation is given by

a.
$$\Delta u = A[\Delta \sigma_3 + B(\Delta \sigma_1 - \Delta \sigma_3)]$$

b.
$$\Delta u = B[\Delta \sigma_3 + A(\Delta \sigma_1 - \Delta \sigma_3)]$$

$$\Delta u = A[\Delta \sigma_3 + B(\Delta \sigma_1 + \Delta \sigma_3)]$$

d.
$$\Delta u = B[\Delta \sigma_3 + A(\Delta \sigma_1 + \Delta \sigma_3)]$$

08. The total settlement of a compressible soil stratum 2 m deep and having a coefficient of volume compressibility of 0.02 cm²/kg under a pressure increment of 2 kg/cm² will be

- a. 2 cm
- b. 4 cm
- c. 8 cm
- d. 10cm

09. The relationship between water content (w%) and number of blows (N) is soils, as obtained from casagrande's liquid limit device is given by

 $w = 20 - \log_{10} N$

the liquid limit of the soil is

- a. 15.6%
- b. 16.6%
- c. 17.6%
- d. 18.6%

10. The results (curves A,B,C and D) of four compaction tests on different soils are shown in Fig 8.3

- 1. Silty sand, modified test 2. Silt sand, standard test
- 3. fat clay, modified test
- Fat clay, standard test
- 5. Curves A, B, C and D correspond respectively to test
- a. 1, 3, 2 and 4
- b. 1, 2, 3 and 4
- c. 2, 1, 3 and 4
- d. 2, 1, 4 and 3

11. The results of a consolidated drained traxial shear test on a normally consolidated clay are shown if fog 8.4



The angle of internal friction is

- a. $sin^{-1}\left(\frac{1}{3}\right)$
- b. $sin^{-1}\left(\frac{1}{2}\right)$
- c. $sin^{-1}\left(\frac{2}{3}\right)$
- d. $sin^{-1}\left(\frac{1}{\sqrt{2}}\right)$
- 12. Which one of the following statements provides the best argument that direct shear tests are not suited for determining shear parameters of a clay soil
- a. Failure plane is not the weakest plane.
- b. Pore pressures developed cannot be measured
- c. Satisfactory strain levels cannot be maintained
- d. Adequate consolidation cannot be ensured.
- 13. Match List I with List II and select the correct answer using the codes given below the lists

List I List II (Pressure distribution (Soil type) For strutted excavation of foundation trench)

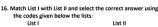
14. Two specimens of clay A and B are tested in a consolidation apparatus.

if $(m_v)_A = 3.6 \times 10^{-4} \text{ m}^2/\text{kN}$ and $(m_v)_B = 1.8 \times 10^{-4} \text{ m}^2 / \text{kN},$ $(C_v)_A = 3.8 \times 10^{-4} \text{ cm}^2 / \text{s},$ $(C_v)_B = 1.9 \times 10^{-4} \text{ cm}^2/\text{s},$ then the ratio K_A/K_B is equal to

- a. 0.0625
- b. 0.25
- c. 1.0
- d. 4.0

15. terzagh's equation of ultimate bearing capacity for a strip footing may be used for square footing resting on pure clay soil with the correction factor

- a. 0.4
- b. 0.6
- c. 1.2
- d. 1.3



List II
(Definition)
Net loading intensity at
which neither soil fails in
shear nor is there any
excessive settlement
The maximum pressure
which soil can carry safely
without risk of shear failu

B. Net safe bearing capacit

C. Safe bearing capacity

Net ultimate bearing capacity divided by factor

D. Allowable bearing pre

capacity divided by factor of safety Minimum gross pressure intensity at the base of foundation at which soil

17. A building is supported on shallow foundation in sand at 1 m below ground level. The water table is at 5 m below the ground surface . For which one of the following foundation will the net bearing capacity of the soil be a maximum

- a. 2 m wide strip footing
- b. 2 m x 2 m square footing
- c. 2 m diameter circular footing
- d. 4 m x 1 m rectangular footing

18. The determination of ultimate bearing capacity on an eccentrically loaded square footing depends upon the concept of useful

- a. Square
- b. Width
- Triangle
- circle

19. Figure 8.6 shows the contact pressure distribution in clayey soil subjected to a uniformly distributed load (udl) through rigid footing (placed on the surface



Which of the following would cause the contact pressu distribution maximum at the centre and decrease towards the outer edge leading to parabolic shape

1. When udl is transmitted through rigid footing placed

- on the surface of a cohesionless soil
 When udl is transmitted through flexible footing
 placed on the surface of a cohesive soil.
 When udl is transmitted through flexible footing
- placed on the surface of a pure clay. ct the correct answer using the codes give

20. A cast-situ bored pile 0.5 m diameter and 10 m deep is placed in a purely cohesive soil. If the cohesion of the soil is 4t/m and adhesion between the pile and the soil is half the value of cohesion, then theu Itimate bearing capacity of the pile is given by

 $\frac{19\pi}{\cdot}$ tonnes

 $\frac{29\pi}{}$ tonnes

tonnes

 $\frac{49\pi}{1}$ tonnes

21. Match List I and List II and select the correct answer the codes given below the lists List I List II

(property of soil) (Laboratory equipment)

A. Grain size

1. pycnometer

